

GENERAL NOTES

1. ALL ELEVATIONS BASED ON TOP OF CONCRETE ELEVATION 0'-0" = 49.0'. VERIFY W/ ARCH
2. COORDINATE CIVIL, PEMB, ARCHITECTURAL AND MEP DETAILS WITH THESE DRAWINGS.
3. ENGINEERING DESIGN REQUIREMENTS:

CONSTRUCTION DOCUMENTS REQUIRED ENGINEERING DESIGN INFORMATION				PROJECT: Future Farmers of America, near Mansura, LA	
IBC 2021 LOADING INFORMATION					
1603.1.1 Floor Live Load	1st floor	100 psf			
	Upper floors	100 psf			
	Stairs, lobbies, corridors	100 psf			
	Corridors above 1st flr	N/A			
	Concentrated anywhere	2000 lbs			
1603.1.2 Roof Live Load					
		20 psf			
1603.1.3 Roof Snow Load					
		5 psf	The ground snow load, P_g		
1603.1.4 Wind Design Data	$V_{ultimate}$ =	112 mph	1	Ultimate design wind speed (3-second gust)	
	V_{des} =	87 mph	1	Nominal design wind speed	
	Risk Category:	II	2		
	Exposure	C	3	Wind exposure Category	
	Enclosed Bldg	0.18	4	Internal pressure coefficient	
	On drawings		5	Components and cladding wind pressures	
1603.1.5 Earthquake Design Data	Risk Category:	II	1		
		1.00	2	Seismic importance factor, I_p	
	S_{ps} =	0.107	3	Mapped spectral response accelerations	
	S_1 =	0.061	3	S_1 and S_2	
		D	4	Site class from soils report	
	S_{ms} =	0.114	5	Spectral response coefficients, S_{ms} , and S_{ms1}	
	S_{ms1} =	0.097	5		
		B	6	Seismic design category	
	Steel Ordinary Braced Frames		7	Basic seismic force-resisting system(s)	
	Per Calculations	0.085	8	Design base shear	
		3-25	9	Seismic response coefficient(s), C_s	
		3-25	10	Response modification factor(s), R	
		Eq Lateral Force Procedure	11	Analysis procedure used	
	1603.1.6 Geotechnical Information				
		1500	Allowable bearing pressure used in design		

4. COORDINATE SIZE, POSITION AND REQUIRED DEPTH OF ALL BLOCK-OUTS FOR SIDING, TRIM, DOORS, ETC.
5. COORDINATE ALL PEMB ANCHOR BOLT SIZES, POSITIONS & REQUIRED PROJECTIONS WITH DETAILS CONTAINED HEREIN.

EARTHWORK & FOUNDATION NOTES:

SOIL REPORT FOR THIS SITE WAS PRODUCED BY GEOTECHNICAL TESTING LABORATORY, REPORT No. 08-25-087, DATED SEPTEMBER 3, 2025.

SITE PREP AND FOUNDATION CONSTRUCTION SHALL BE OBSERVED BY GEOTECH ENGINEER OR DESIGNATED REPRESENTATIVE.

FOUNDATION SUBGRADE PREPARATION:

TO PREPARE FOR FOUNDATION AND SOIL SUPPORTED FLOOR SLAB CONSTRUCTION, ALL TOPSOIL, VEGETATION, ROOTS, AND ANY SOFT SOILS IN THE BUILDING AREA BE STRIPPED FROM THE SITE AND EITHER PROPERLY DISPOSED OR STOCKPILED FOR LATER USE IN LANDSCAPING. UTILITIES SHALL BE LOCATED AND REROUTED AS NECESSARY.

ANY TREES OR TREE STUMPS LOCATED WITHIN THE BUILDING PAD SHALL BE GRUBBED AND REMOVED. THE DIAMETER OF THE EXCAVATION SHALL BE AT LEAST THREE (3) FEET LARGER THAN THE TREE DIAMETER AND DRY SOILS AND ROOTS ½ INCH IN DIAMETER OR GREATER SHALL BE GRUBBED TO A MINIMUM DEPTH OF FOUR (4) FEET BELOW FINISHED SUBGRADE ELEVATION. THE RESULTING DEPRESSION SHALL BE BACKFILLED AND COMPACTED AS REQUIRED IN THE SELECT FILL SECTION OF THIS REPORT.

TO REMEDIATE THE VARIABLE SOIL CONDITIONS IN THE SURFICIAL ZONE AND PROVIDE A CONSISTENT SUBGRADE FOR SLAB SUPPORT, A UNIFORM LAYER OF DENSITY-APPROVED SELECT FILL BE PROVIDED BENEATH THE FLOOR SLAB. AFTER STRIPPING THE SITE, THE BUILDING PAD SHALL BE CUT TO AN ELEVATION WHICH ALLOWS THE PLACEMENT OF AT LEAST TWO (2) FEET OF DENSITY-APPROVED SELECT FILL BELOW THE FINAL SUBGRADE ELEVATION FOR THE FLOOR SLAB. THE SELECT FILL BUILDING PAD SHALL EXTEND AT LEAST FIVE (5) FEET BEYOND THE EDGE OF THE BUILDING.

AFTER STRIPPING AND UNDERCUTTING AS REQUIRED HEREIN, THE BUILDING AREA SHALL BE PROOF-ROLLED WITH A HEAVY, LOADED PNEUMATIC-TIRED VEHICLE SUCH AS A 20 TO 25 TON LOADED DUMP TRUCK. ALL AREAS BENEATH THE FLOOR SLAB BE PROOF-ROLLED TO IDENTIFY LOOSE OR SOFT SOILS. ALL PROOF-ROLLING AND UNDERCUTTING ACTIVITIES SHALL BE WITNESSED BY THE GEOTECHNICAL ENGINEER OR AUTHORIZED REPRESENTATIVE AND SHALL BE PERFORMED DURING A PERIOD OF DRY WEATHER. ANY WEAK AREAS WHICH YIELD UNDER THE PROOF-ROLL, OR ANY AREAS WITH A TENDENCY TO PUMP SHALL BE MITIGATED. SUCH MITIGATION MAY INCLUDE OVER-EXCAVATION AND BACKFILLING, REPROCESSING TO REMOVE MOISTURE, MODIFICATION WITH LIME OR CEMENT ADMIXTURE, OR USING GEOTEXTILES. IN THE EVENT SUCH MITIGATION IS REQUIRED, THE GEOTECHNICAL ENGINEER SHALL BE CONTACTED TO DESIGN AN APPROPRIATE PROCEDURE.

AFTER PROOF-ROLLING BUT PRIOR TO PLACING FILL, THE EXPOSED SOILS SHALL BE SCARIFIED AND THEN PROCESSED TO A MOISTURE CONTENT BETWEEN ONE (1) PERCENTAGE POINT BELOW AND THREE (3) PERCENTAGE POINTS ABOVE THE STANDARD PROCTOR OPTIMUM. THE SUBGRADE SOILS SHALL BE COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF THE STANDARD PROCTOR (ASTM D-698) MAXIMUM DRY DENSITY FOR A DEPTH OF AT LEAST EIGHT (8) INCHES BELOW THE SURFACE.

SELECT FILL:

AFTER THE SUBGRADE HAS BEEN PREPARED AND INSPECTED, FILL PLACEMENT MAY BEGIN. SELECT FILL MATERIAL SHALL BE FREE OF ORGANIC OR OTHER DELETERIOUS MATERIALS, HOMOGENEOUS MIXTURE, HAVE A MAXIMUM PARTICLE SIZE OF THREE (3) INCHES, HAVE A LIQUID LIMIT LESS THAN 40 AND PLASTICITY INDEX BETWEEN 8 AND 20, AND CONSIST OF SILTY-CLAYEY SANDS (SM-SC), LOW PLASTICITY SANDY CLAYS (CL), OR CLAYEY SANDS (SC) AS DEFINED BY THE UNIFIED SOIL CLASSIFICATION SYSTEM. IN ADDITION TO THE ABOVE REQUIREMENTS, THE MATERIAL SHALL HAVE A MINIMUM OF 30 PERCENT RETAINED ON THE NO. 200 SIEVE. THE ON-SITE SURTICIAL SOILS DO NOT MEET THE REQUIREMENT FOR USE AS SELECT FILL ON THIS PROJECT. IF A FINE-GRAINED MATERIAL IS USED FOR FILL, VERY CLOSE MOISTURE CONTENT CONTROL WILL BE REQUIRED TO ACHIEVE THE REQUIRED DEGREE OF COMPACTION.

FILL SHALL BE PLACED IN MAXIMUM LIFTS OF EIGHT (8) INCHES OF LOOSE MATERIALS AND SHALL BE COMPACTED WITHIN THE RANGE OF ONE (1) PERCENTAGE POINT BELOW TO THREE (3) PERCENTAGE POINTS ABOVE THE OPTIMUM MOISTURE CONTENT VALUE AND A MINIMUM OF 95 PERCENT OF THE MAXIMUM DENSITY AS DETERMINED BY THE STANDARD PROCTOR (ASTM D-698) TEST. IF WATER MUST BE ADDED, IT SHALL BE UNIFORMLY APPLIED AND THOROUGHLY MIXED INTO THE SOIL BY DISKING OR SCARIFYING.

EACH LIFT OF COMPACTED SOIL SHALL BE TESTED AND INSPECTED BY THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE PRIOR TO PLACEMENT OF SUBSEQUENT LIFTS. IT IS REQUIRED THAT FIELD DENSITY TESTS BE TAKEN AT A FREQUENCY OF NOT LESS THAN ONE (1) TEST PER 2,500 SQUARE FEET OF SURFACE AREA PER LIFT OR A MINIMUM OF FOUR (4) PER LIFT FOR EACH TESTED AREA FOR THE BUILDING. THE FILL SHOULD BE USED TO ELEVATE THE BUILDING PAD SO THAT POSITIVE DRAINAGE IS PROVIDED AWAY FROM THE BUILDING.

SHALLOW FOOTINGS:

ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY GEOTECHNICAL ENGINEER OR AN AUTHORIZED REPRESENTATIVE PRIOR TO STEEL AND CONCRETE PLACEMENT TO ASSESS WHETHER THE FOUNDATION MATERIALS APPEAR CONSISTENT WITH THE BORING LOGS. SOFT OR LOOSE SOIL ZONES ENCOUNTERED AT THE BOTTOM OF THE FOOTING EXCAVATIONS SHALL BE REMOVED AND THE CAVITY SHALL BE BACKFILLED WITH COMPACTED SELECT FILL, FLOWABLE GROUT FILL, CRUSHED STONE FLEXIBLE BASE, CONCRETE, OR OTHER APPROVED MATERIAL AND PLACEMENT CONTROL.

UTILITIES WHICH PROJECT THROUGH THE SLAB ON GRADE SHALL BE DESIGNED WITH EITHER SOME DEGREE OF FLEXIBILITY OR WITH SLEEVES. SUCH DESIGN FEATURES WILL HELP REDUCE DAMAGE TO UTILITY LINES IF VERTICAL MOVEMENTS OCCUR.

CONCRETE NOTES

1. ALL CONCRETE WORK TO BE IN ACCORDANCE WITH ACI 301 AND ALL RELATED ACI & ASTM REFERENCES CONTAINED THEREIN. SEE ALSO SPECIFICATIONS FOR SPECIFIC ADMIXTURE REQUIREMENTS FOR BASEMENT CONCRETE.
2. DEFORMED REINFORCING STEEL SHALL BE ASTM A615 GRADE 60.
3. ALL CONCRETE IS NORMAL WEIGHT.
4. CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4000 PSI.
5. CEMENT – CONFORMING TO THE FOLLOWING:
5.1. ASTM C150, TYPE I PORTLAND, GREY COLOR.
5.2. FLY ASH, ASTM C 618 CLASS C MAY BE USED AS A PARTIAL REPLACEMENT, NOT EXCEEDING 25% BY WEIGHT, FOR TYPE I CEMENT.
6. SLAB ON GRADE CONCRETE SHALL MEET ACI 301 TABLE 4.2.2.1 FOR MINIMUM CEMENTITIOUS MATERIAL CONTENT.
7. FINE AND COARSE AGGREGATES: ASTM C33.
8. WATER: CLEAN AND NOT DETRIMENTAL TO CONCRETE.
9. SUBMIT CONCRETE CURING METHOD FOR APPROVAL. ASTM C309 CURING COMPOUND IS ACCEPTABLE. CHECK WITH ARCH FLOOR COVERING ANY SPECIFIC REQUIREMENTS THAT MAY BE AFFECTED BY CURING METHOD.
10. AIR ENTRAINMENT ADMIXTURE: ASTM C260.
11. SLUMP: 4" AT THE POINT OF DELIVERY, TOLERANCE ±1".
12. USE APPROVED HIGH OR MID-RANGE WRDA TO ACHIEVE HIGHER SLUMP IN CONCRETE.
13. WATER REDUCING ADMIXTURE: ASTM C494 TYPE A; AND SAME MANUFACTURERS AS ABOVE WHEN USED TOGETHER.
14. DO NOT ADD WATER AT JOB SITE WITHOUT PRIOR APPROVAL FROM DESIGN TEAM.
15. PROVIDE CORNER BARS AT ALL CORNERS AND T-INTERSECTIONS OF WALLS EQUAL TO HORIZONTAL REINFORCEMENT.
16. OPENINGS IN CONCRETE SHALL HAVE SUPPLEMENTARY REINFORCING, SEE DETAILS IN DRAWINGS.
17. PROVIDE REBAR SPlice LENGTHS AS REQUIRED BY ACI 318 AND THE SPECIFIED COMPRESSIVE STRENGTH OF THE VARIOUS CONCRETE MIXES.
15. PROVIDE (2)-#4 REINFORCEMENT BARS x 2'-0" AT RE-ENTRANT CORNERS AND AROUND RECTANGULAR HOLES IN SLABS UNLESS NOTED OTHERWISE. PLACE BARS DIAGONALLY TO CORNER WITH 1" CLEARANCE FROM THE TOP OF THE SLAB AT THE CORNER.
16. AT CONTRACTOR'S OPTION, FOOTING AND GRADE BEAM FACES NOT EXPOSED TO VIEW NEED NOT BE FORMED.
17. REINFORCING STEEL DOWELED INTO EXISTING CONCRETE SHALL BE PLACED IN PROPERLY PREPARED DRILLED HOLES IN EPOXY PRODUCT TO ENGINEER APPROVAL.
18. EPOXY ADHESIVE FOR ANCHORAGE IN CONCRETE: ASTM C881, TWO-COMPONENT MATERIAL SUITABLE FOR DRY OR DAMP SURFACES. PROVIDE MATERIAL TYPE, GRADE, AND CLASS TO SUIT REQUIREMENTS. SUBMIT PRODUCT TO DESIGN TEAM FOR REVIEW. STRICTLY FOLLOW MANUFACTURER'S 'S DIRECTIONS FOR PREPARATION AND USE.
19. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT; REINFORCING BARS SHALL BE PLACED AND TIED IN THE FORMS TO ACHIEVE CLEARANCES IN STRICT ACCORDANCE WITH THE REQUIREMENTS OF ACI-318, BUILDING CODE AND COMMENTARY (LATEST EDITION), AND SHALL MEET THE FOLLOWING CRITERIA:

CONCRETE CAST AGAINST & EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO WEATHER (#6 & LARGER)	2"
CONCRETE EXPOSED TO WEATHER (#5 & SMALLER)	1½"
CONCRETE NOT EXPOSED TO WEATHER (#11 & SMALLER)	1"

20. SOME AMOUNT OF MINOR CRACKING IS TO BE EXPECTED IN ANY CONCRETE WORK. CONCRETE SHRINKAGE WILL ALWAYS CAUSE SOME AMOUNT OF CRACKING. REINFORCING STEEL HAS BEEN DESIGNED AND DRAWINGS SHOW THE PLACEMENT TO LIMIT ANY CRACK WIDTHS TO AN ACCEPTABLE WIDTH. THE DESIGNER IS NOT RESPONSIBLE FOR MINOR CRACKING IN THE CONCRETE WORK THAT DOES NOT AFFECT THE STRENGTH OR SERVICEABILITY OF THE STRUCTURE OF WHICH THE CONCRETE IS A PART.
21. SAMPLING AND TESTING FOR QUALITY CONTROL DURING CONCRETE PLACEMENT SHALL BE PER "COMPRESSIVE STRENGTH TESTS" ASTM C 39; ONE SET FOR EACH DAY'S POUR EXCEEDING 5 YD. PLUS ADDITIONAL SETS FOR EACH 50 CU. YD. MORE THAN THE FIRST 25 CU. YD. OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY; ONE SPECIMEN TESTED AT 7 DAYS, TWO SPECIMENS TESTED AT 28 DAYS, AND ONE SPECIMEN RETAINED IN RESERVE FOR LATER TESTING IF REQUIRED.

STEEL NOTES

1. ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL BUILDINGS AND BRIDGES, AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, APPLICABLE PORTIONS OF OSHA 29 CFR PART 1910 AND PART 1926, AND ANY OTHER APPLICABLE FEDERAL, STATE OR LOCAL REGULATIONS.
2. STRUCTURAL STEEL: ASTM A992 FOR W & WT SHAPES, ASTM A36 FOR PLATES, CHANNELS & ANGLES.
3. STRUCTURAL STEEL TUBE (HSS) SHALL BE ASTM A500, GRADE B OR C.
4. BOLTED CONNECTIONS: ASTM F3125 GRADE A325-N TYPE 1, BEARING TYPE, SNUG-TIGHTENED. ALL BEAMS SHALL BE EQUALLY SPACED BETWEEN COLUMN CENTER LINES UNLESS DIMENSIONED OTHERWISE.
5. UNLESS OTHERWISE SHOWN ON THE DESIGN DRAWINGS, ALL FIELD CONNECTIONS SHALL BE BOLTED AND ALL SHOP CONNECTIONS SHALL BE EITHER BOLTED OR WELDED.
6. FABRICATE/ERECT IN ACCORDANCE WITH AISC SPECIFICATIONS AND CODE OF STANDARD PRACTICE.
7. ALL STEEL TO BE GALVANIZED, SEE NOTES.
8. METAL FLOOR SHALL BE INSTALLED IN ACCORDANCE WITH STEEL DECK INSTITUTE AND MANUFACTURER'S RECOMMENDATIONS.
9. METAL FLOOR DECK SHALL BE GALVANIZED (G90), MANUFACTURED & INSTALLED PER MFG'S SHOP DRAWING. DECK TYPE AND GAUGE ARE GIVEN IN DRAWINGS
10. ALL WELDING SHALL BE IN ACCORDANCE WITH THE LATEST AISC AND AWS SPECIFICATIONS USING E70 ELECTRODES. WELDS SHALL BE IN ACCORDANCE WITH AWS D11.

MINIMUM FILLET WELD SIZES (IN)	
MATL THKNS OF THICKER PART JOINED	MIN WELD SIZE
UP TO ¼" INCLUSIVE	⅝"
OVER ¼" TO ½"	¾"
OVER ½" TO ¾"	⅞"
OVER ¾" TO 1½"	1"
THE MAXIMUM WELD SIZE IS LIMITED TO THE THICKNESS OF THE THINNER PART IN JOINT.	

11. COMPOSITE METAL DECK SHALL BE GALVANIZED, MANUFACTURED & INSTALLED PER SPECIFICATION ABOVE & PER DECK MANUFACTURER'S INSTALLATION INSTRUCTIONS.
12. METAL DECK MANUFACTURER QUALIFICATIONS: MEMBER OF THE STEEL DECK INSTITUTE.
13. COMPOSITE DECK SHALL BE MULTI-SPAN AND SHALL NOT REQUIRE SHORING DURING THE CONCRETE PLACEMENT PROCEDURE.
17. ATTACHMENT, COMPOSITE DECK: SEE SPECIFICATION & SHOP DRAWINGS.
18. SHEAR STUDS ARE SPECIFIED IN COMPOSITE DECK DETAIL.

Walls	Effective Area (ft²)	Pressure (psf)				
		Zone 4		Zone 5		
		Positive	Negative	Positive	Negative	
Components and Cladding	10	32	-34	32	-42	
	50	28	-31	28	-36	
	100	27	-29	27	-33	
	250	25	-28	25	-29	
	500	23	-26	23	-26	

WALLS

STRENGTH LEVEL (ULTIMATE)
COMPONENT AND CLADDING WIND
LOADING ON WALL AREAS
(a=6 FT)

STRENGTH LEVEL (ULTIMATE) COMPONENT AND CLADDING WIND LOADING ON WALL AREAS (a=6 FT)

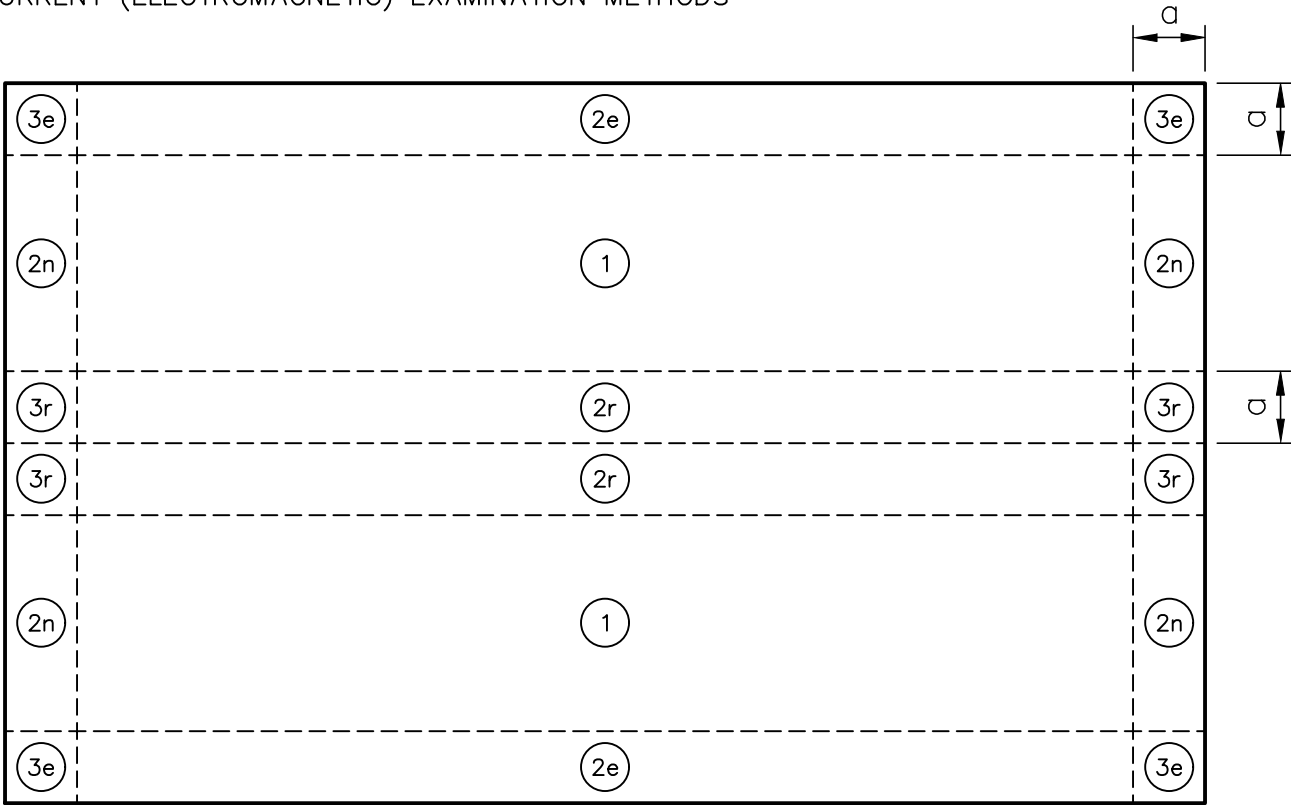
GALVANIZING NOTES

UNLESS OTHERWISE NOTED, ALL EXTERIOR STEEL MATERIALS SHALL BE GALVANIZED.

1. GALVANIZING OF STEEL SHAPES, PLATES, AND HARDWARE SHALL BE IN ACCORDANCE WITH THE FOLLOWING ASTM SPECIFICATIONS:
1.1. STEEL SHAPES AND PLATES – ASTM A123
1.2. ASTM A325/A325M BOLTS AND CORRESPONDING NUTS AND WASHERS ASTM B695 OR ASTM F2329
1.3. ASTM A36/A36M THREADED BAR OR ASTM A307 BOLTS AND CORRESPONDING NUTS AND WASHERS – ASTM F2329 OR ASTM B695
2. FABRICATION
2.1. IT SHALL BE THE FABRICATOR'S RESPONSIBILITY TO SAFEGUARD AGAINST EMBRITTLEMENT AND WARPAGE IN ACCORDANCE WITH ASTM A143 AND ASTM A384. FABRICATION DETAILS SHALL BE IN ACCORDANCE WITH ASTM A385 TO ALLOW FOR THE CREATION OF HIGH QUALITY ZINC COATINGS. IF PRACTICAL, CUTTING, DRILLING AND WELDING SHALL BE PERFORMED BEFORE GALVANIZING.
2.2. WELD SLAG SHALL BE REMOVED BEFORE GALVANIZING.
2.3. THE EDGES OF TIGHTLY CONTACTING SURFACES SHALL BE COMPLETELY SEAL WELDED.
2.4. VENT HOLES SHALL BE PROVIDED FOR PIPING OR TUBULAR ASSEMBLIES AS REQUIRED BY ASTM A385.
2.5. POTENTIAL PROBLEMS THAT REQUIRE A MODIFICATION IN DESIGN SHALL BE BROUGHT TO THE ATTENTION OF THE PURCHASER BEFORE PROCEEDING.
3. GALVANIZING OF STEEL HARDWARE
3.1. NUTS SHALL BE TAPPED OVERSIZE IN ACCORDANCE WITH ASTM A563/A563M.
3.2. NUT THREADS SHALL BE RETAPPED AFTER HOT-DIP GALVANIZING TO PROVIDE PROPER FIT.
3.3. DIRECT TENSION INDICATORS, IF USED, SHALL BE MECHANICALLY GALVANIZED BY THE INDICATOR MANUFACTURER IN ACCORDANCE WITH CLASS 50 OF ASTM B695.
4. REPAIR
4.1. ANY DAMAGE TO GALVANIZING SHALL BE REPAIRED IN ACCORDANCE WITH ASTM A780.
4.2. BEFORE REPAIR OF DAMAGED GALVANIZED COATING, EXPOSED SUBSTRATE METAL SHALL BE CLEANED TO BRIGHT METAL AND FREE OF ALL VISUAL RUST, OIL, OR GREASE. ANY NONADHERING GALVANIZING SHALL BE REMOVED TO THE EXTENT THAT THE SURROUNDING GALVANIZING IS INTEGRAL AND ADHERENT.
4.3. IF SURFACE DEFECTS EXCEED 2% OF A MEMBER'S AREA, THE DEFECTS SHALL BE REPAIRED BY REDIPPING THE MEMBER IN THE ZINC BATH.
4.4. COLD REPAIR USING AN ORGANIC ZINC RICH COATING SHALL BE PERMITTED IF THE FOLLOWING CONDITIONS EXIST:
4.4.1. TOTAL DAMAGED AREA IS LESS THAN 1% OF THE TOTAL COATED AREA OF THE MEMBER BEING REPAIRED
4.4.2. NO SINGLE REPAIR IS GREATER THAN 2 SQUARE INCHES (1300 MM2)
4.4.3. NO SINGLE REPAIR IS GREATER THAN 12 INCHES (300 MM) LONG.
4.4.4. FOR COATING APPLIED FOR A COLD REPAIR, THE DRY FILM THICKNESS SHALL BE 2 TO 3 MILS (0.05 MM TO 0.08 MM) AND CONTAIN A MINIMUM OF 65% ZINC DUST BY WEIGHT.

REFERENCES

- ASTM A143/A143M – STANDARD PRACTICE FOR SAFEGUARDING AGAINST EMBRITTLEMENT OF HOT-DIP GALVANIZED STRUCTURAL STEEL PRODUCTS AND PROCEDURE FOR DETECTING EMBRITTLEMENT
- ASTM A384/A384M – STANDARD PRACTICE FOR SAFEGUARDING AGAINST WARPAGE AND DISTORTION DURING HOT-DIP GALVANIZING OF STEEL ASSEMBLIES
- ASTM A385/A385M – STANDARD PRACTICE FOR PROVIDING HIGH-QUALITY ZINC COATINGS (HOT-DIP)
- ASTM A780/A780M – STANDARD SPECIFICATION FOR REPAIR OF DAMAGED AND UNCOATED AREAS OF HOT-DIP GALVANIZED COATINGS
- ASTM B695 – STANDARD SPECIFICATION FOR COATINGS OF ZINC MECHANICALLY DEPOSITED ON IRON AND STEEL
- ASTM E376 – STANDARD PRACTICE FOR MEASURING COATING THICKNESS BY MAGNETIC-FIELD OR EDDY-CURRENT (ELECTROMAGNETIC) EXAMINATION METHODS



ROOF PLAN

Roof	Effective Area (ft²)	Pressure (psf)											
		Zone 1		Zone 2e		Zone 2n		Zone 2r		Zone 3e		Zone 3r	
		Positive	Uplift	Positive	Uplift	Positive	Uplift	Positive	Uplift	Positive	Uplift	Positive	Uplift
Components and Cladding	10	19	-58	19	-58	19	-85	19	-85	19	-85	19	-101
	100	16	-35	16	-35	16	-58	16	-58	16	-58	16	-67
	500	16	-18	16	-18	16	-47	16	-47	16	-47	16	-53
	250	16	-18	16	-18	16	-31	16	-31	16	-31	16	-53
	500	16	-18	16	-18	16	-31	16	-31	16	-31	16	-53
STRENGTH LEVEL (ULTIMATE) COMPONENT & CLADDING WIND LOAD PRESSURES ON ROOF AREAS													
WIND DESIGN CRITERIA ASCE 7-16: 112 MPH, EXP C, RC II													
ALL ROOF COMPONENTS TO BE DESIGNED FOR UPLIFT PRESSURES SHOWN IN TABLE a = 6 FEET													

DRAWING INDEX				
DWG. NO.	TITLE	DATE ISSUE	REV	ISSUED HEREIN
S1.0	GENERAL NOTES & COLUMN SCHEDULE	MARCH 2026	0	✓
S1.1	FOUNDATION PLAN	MARCH 2026	0	✓
S1.2	FOUNDATION DETAILS	MARCH 2026	0	✓
S2.1	LEVEL ONE WALL FRAMING	MARCH 2026	0	✓
S2.2	2ND FLOOR FRAMING PLAN	MARCH 2026	0	✓
S2.3	STRUCTURAL STEEL DETAILS	MARCH 2026	0	✓
S3.1	STAIR DETAILS	MARCH 2026	0	✓

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STATE OF LOUISIANA
THOMAS
REG. NO. 10026
PROFESSIONAL ENGINEER
CIVIL ENGINEERING

Project No. 0225

Drawn By: PHH

Approved By: TAW

Preliminary Date: MARCH 31, 2026

Release Date: MARCH 31, 2026

File:

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S1.0

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